

## GEO TECHNICAL INVESTIGATION REPORT

REPORT No.: **GT / 3485 / 2024 – 25** 

PROJECT: Proposed Construction of (S+5) & (S+4)

**Buildings at Alwal, Hyderabad** 

CLIENT: M/s Avison Properties LLP

DURATION: February 2025

GEOTECHNICAL CONSULTANTS: GEO TECHNOLOGIES

**ISO 9001:2015 COMPANY** 

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#### 1. INTRODUCTION

M/s Avison Properties LLP have engaged M/s GEO TECHNOLOGIES as consultant for Soil investigation for the proposed Construction of (S+5) & (S+4) Floors Buildings at Alwal, Hyderabad.

Soil investigation was conducted by drilling two (2) boreholes, conducting standard Penetration Tests, and lab testing of soil and rock samples.

The results of this investigation and recommendations are presented in this Report. All the investigations are conducted in accordance with the relevant IS Codes.

#### 2. FIELD INVESTIGATIONS

#### **DRILLING:**

The bore holes were drilled in the site at the locations specified by the client (Fig.1). Rotary Drilling was performed as per IS: 1892. The size of the casing used was 150 / 90 mm.

The following information was collected during the drilling operations:

- Nature of strata - Details of soil samples

- Colour of Return Water - Rate of drilling

### STANDARD PENETRATION TEST (SPT):

SPT were conducted at 1.5 m depth intervals in soil, in accordance with IS: 2131-1981. Soil samples were extracted from the SPT sampler and preserved in polythene bags.

#### **SAMPLES**:

Soil samples were collected as per IS: 2132. All the samples collected from the bore holes were properly packed, labeled and transported to Geo Technologies Soil Testing Laboratory at Hyderabad.

#### **FIELD BORE LOGS:**

All the details collected from the field operations are presented in Logs of Bore holes in Annexure-1 at the end of this Report. These logs contain depth wise strata details, depth and type of soil & rock samples collected, results of Standard Penetration Tests, and color of return water etc.



#### 3. LABORATORY TESTING

The following tests were performed on the Soil samples:

- Natural Moisture Content
- Specific gravity (IS: 2720: part3 1980)
- Grain size distribution (IS 2720 part 4 1985)

No cores were recovered in SDR strata.

#### 4. RESULTS

Table 1 gives the results of lab tests of soil samples.

Fig. 1 gives the site plan showing the locations of boreholes.

Fig. 2 gives the combined Log of bore holes.

Appendix gives the calculations for SBC.

Annexure-1 gives the field bore log charts.

Annexure-2 gives the BIS Codes.

# 5. SUBSOIL PROFILE

The subsoil profile in the boreholes is as follows:

Depth, m	Strata	N
0.0 – 1.5	Filling / silty clay	-
1.5 – 3.0	Silty gravel	21 – 35
3.0 – 10.0	Soft Disintegrated Rock (SDR)	>50

At the time of drilling, no water table was seen in the boreholes.



#### 6. RECOMMENDATIONS

The following recommendations are made for the proposed (S+5) & (S+4) Floors Buildings at Alwal, Hyderabad. These are based on drilling of 2 boreholes.

1. The detailed subsoil profiles each borehole are given in bore log diagram (Fig.2). The subsurface profile in the site can be generalized as follows:

Depth, m	Strata	N
0.0 – 1.5	Filling / silty clay	-
1.5 – 3.0	Silty gravel	21 – 35
3.0 – 10.0	Soft Disintegrated Rock (SDR)	>50

- 2. No water table was encountered till borehole termination depth.
- 3. Open foundations are recommended at 2.0 / 3.0 m depth below EGL
- 4. SBC is recommended as follows:

Depth, m	Strata	SBC, t/m²				
2.0	Silty gravel	25				
3.0	Soft Disintegrated Rock (SDR)	35				

- 5. This is based on the assumption of footing width of 2 m. Actual shape and size will be based on the loads from the superstructure.
- 6. All loose / isolated boulders at foundation level should be fully removed and the
- 7. As per IS Code of Practice 2950 (Part -1) 1981, Reaffirmed, 2008, App B: Subgrade reaction modulus (k-value) is recommended as 5 kg/cm<sup>2</sup>/cm for dry conditions.

#### For **GEO TECHNOLOGIES**

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# Project: Proposed (S+5) & (S+4) Floors Buildings at Alwal, Hyderabad

TABLE-1: Results of Lab testing of Soil samples

					Grain size, %						
BH No.	Depth, m	Soil	NMC%	Sp. gr.	Gr >4.75 mm	Sa 4.75 to 0.075 mm	Fines (Si+CI) <0.075 mm				
BH-1	1.5	Silty gravel	2.1	-	41	36	23				
BH-2	1.5	Silty gravel	2.2	-	45	35	20				

# **Appendix: Calculations for SBC**

#### <u>Isolated Footings in Silty gravel at 2.0 m depth:</u>

### a) Shear criterion:

As per Teng's equation, Net Safe bearing capacity based on shear failure criterion (with F.S. = 3.0) is given as:

$$q (safe) = 1/18 [2 N^2 B Rr + 6 (100 + N^2) D Rq] kN/m^2$$

B = width of Footing, assumed as 2 m, D = depth of foundation = 1.0 m (Effective),

N = corrected N value = 20

Rq & Rr = correction factors for water table = 0.5

 $q (safe) = 255 kN/m^2$ .

# b) Settlement Criterion:

For a settlement of 40 mm (N = 20, B = 3 m)

q (Net Allowable) = 12.25 N [(B + 0.3) / B] = 281 KN/ m<sup>2</sup>.

Recommended net safe bearing pressure for Isolated footings of width 2 m resting in silty gravel at 2 m below EGL is 25 t/m<sup>2</sup> for allowable settlement of 40 mm.



# Isolated Footings in SDR at 3.0 m depth:

# a) Shear criterion:

As per Teng's equation, Net Safe bearing capacity based on shear failure criterion (with F.S. = 3.0) is given as:

q (safe) = 
$$1/18 [2 N^2 B Rr + 6 (100 + N^2) D Rq] kN/m^2$$

B = width of Footing, assumed as 2 m, D = depth of foundation = 2.0 m,

N = corrected N value = 35

Rq & Rr = correction factors for water table = 0.5

 $q (safe) = 577 kN/m^2$ .

# b) Settlement Criterion:

For a settlement of 40 mm (N = 35, B = 2 m) q (Net Allowable) =  $12.25 \text{ N} [(B + 0.3) / B] = 493 \text{ KN/ m}^2$ .

Recommended net safe bearing pressure for Isolated footings of width 2 m resting in SDR at 3 m below EGL is 45 t/m<sup>2</sup> for allowable settlement of 40 mm.

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# Annexure-1

# **FIELD BORE CHARTS**

FIG:1 SITE PLAN SHOWING THE LOCATION OF BORE HOLE

Project: Avisun Properties Alwal Feb-2024

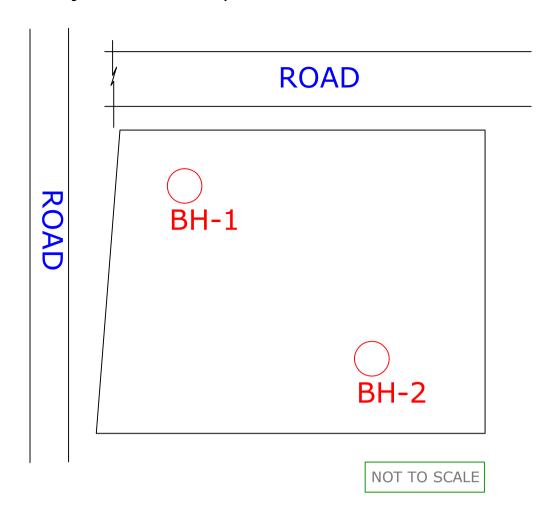
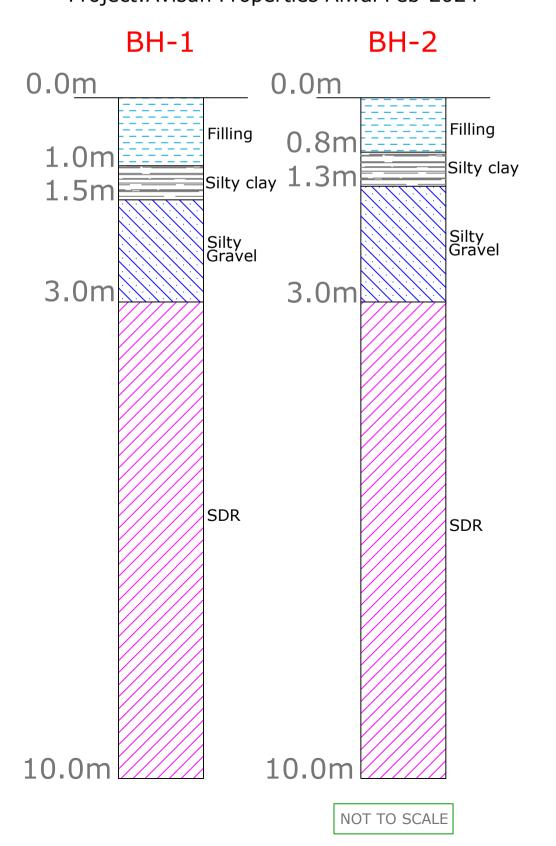


FIG:2 LOG OF BORE HOLES
Project: Avisun Properties Alwal Feb-2024



# **GEO TECHNOLOGIES**

							FIELD E	BORE	LOG CHAR	Γ							
Project: Proposed Construction of (S+5) & (S+4) Buildings at Alwal, Hyderabad Date: 16-02-2025								ВО	RE HOLE NO. 1	Ground Level: Dia. Of Casing: NX Water Table:							
	pth				Sam	pling	SPT		D	etails of Ro							
(r	n)	Length of Run		of	Log of Bore	Depth		No. blows for Penetration of	N	>10cm core	Total	No. of	% of core	RQD Value %	Avg. RQD %	Water colour	Rate of Drill Min/m
From	То	(m)			(m)		(15-30-45)cm	Value	Pieces(cm)	Length (cm)	Pieces	Recovery		/0		IVIII/III	
0.0	1.0	1.0	Filling		1.00	D/S											
1.0	1.5	0.5	Silty clay		1.50	SPT	7-8-13	21							Reddish		
1.5	3.0	1.5	Silty Gravel		2.50 3.00	D/S SPT	22cm/50blows	>50									
3.0	4.5	1.5			4.00 4.50	D/S SPT	20cm/50blows	>50									
4.5	6.0	1.5			5.50 6.00	D/S SPT	16cm/50blows	>50							Prownich		
6.0	7.5	1.5	SDR		7.00 7.50	D/S SPT	13cm/50blows	>50							Brownish		
7.5	9.0	1.5			8.50 9.00	D/S SPT	12cm/50blows	>50									
9.0	10.0	1.0			10.00	D/S											

SDR=Soft Disintigrated Rock

cm/50= no.of blows

sp=Small pieces vsp=Very Small Pieces

	FIELD BORE LOG CHART																																							
Project: Proposed Construction of (S+5) & (S+4) Buildings at Alwal, Hyderabad Date: 16-02-2025								ВО	RE HOLE NO. 2	Ground Level: Dia. Of Casing: NX Water Table:																														
De	pth				Sam	pling	SPT		De	etails of Ro	ck core	1																												
(n	n)	Length of Run	Description	Log of Bore	Depth	Туре	No. blows for Penetration of	N	>10cm core	Total	No. of	% of core	RQD Value %	Avg. RQD %	Water colour	Rate of Drill Min/m																								
From	То	(m)			(m)		(15-30-45)cm	Value	Pieces(cm)	Length (cm)	Pieces	Recovery			IVIIII/ITI																									
0.0	0.8	0.8	Filling												-																									
8.0	1.3	0.5	Silty clay		1.00	D/S									Reddish																									
1.3	1.5	0.2	Cilty Croyel		1.50	SPT	10-15-20	35																																
1.5	3.0	1.5	Silty Gravel																	2.50 3.00	D/S SPT	24cm/50blows	>50																	
3.0	4.5	1.5																						E,	E	E,	E,		4.00 4.50	D/S SPT	19cm/50blows	>50								
4.5	6.0	1.5																										5.50 6.00	D/S SPT	17cm/50blows	>50							Brownish		
6.0	7.5	1.5	SDR	SDR	SDR	SDR	SDR	SDR	SDR	SDR		7.00 7.50	D/S SPT	14cm/50blows	>50																									
7.5	9.0	1.5			8.50 9.00	D/S SPT	12cm/50blows	>50																																
9.0	10.0	1.0			10.00	D/S								_		_																								

SDR=Soft Disintigrated Rock

cm/50= no.of blows

sp=Small pieces vsp=Very Small Pieces

#### **ANNEXURE - 2: IS CODES**

- IS: 1892 2021: Subsurface Investigations for Foundations Code of Practice (Second Revision)
- 2. IS: 2131 1981 (Reaffirmed 2021): Method of Standard Penetration Test for Soils.
- 3. IS: 2132 1986: Code of Practice for thin walled tube sampling of Soils.
- 4. IS: 4464 2020: Code of Practice for presentation of drilling information and core description in foundation investigations (Second Revision).
- 5. IS: 2720 Part 4 1985: Methods of Test for Soils Part 4: Grain size analysis.
- IS: 1498 1970: Classification and Identification of Soils for General Engineering Purpose.
- 7. IS: 2720 Part 29 1975: Methods of Test for Soils Part 29: Determination of dry density of soils by core-cutter method.
- IS: 2720 Part 2 1973: Methods of Test for Soils Part 2: Determination of water content.
- 9. IS: 2720 Part 13 1986: Methods of Test for Soils Part 13: Direct shear test.
- IS: 2720 Part 2 1985: Methods of Test for Soils Part 2: Determination of water content.
- 11. IS: 2720 Part 3 / section 2 1980: Methods of Test for Soils Part 3: Determination of Specific gravity; Section 2: Fine, Medium and Coarse Grained Soils.
- 12. IS: 2720 Part 7 1980: Methods of Test for Soils Part 7: Determination of water content Dry density relation using light compaction .
- 13. IS: 2720 Part 16 1979: Methods of Test for Soils Part 16: Laboratory Determination of CBR.
- 14. IS: 9143 1979: Method for determination of unconfined compressive strength of rock materials.
- 15. IS: 8764 1998 Reaffirmed 2008: Method for determination of Point Load Strength Index of rocks.
- 16. IS: 11315 (Part 2) 1987: Method for Quantitative Description of Discontinuities in Rock Mass Part 2: Spacing.
- 17. IS: 11315 (Part 11) 1985: Method for Quantitative Descriptions of Discontinuities in Rock Masses Part 11: Core Recovery and Rock Quality Designation.
- IS: 11315 (Part 12) 1992: Quantitative Description of Discontinuities in Rock Mass Methods – Part 12: Drill Core study.
- 19. IS: 12070 1987: Code of Practice for Design and Construction of Shallow Foundations on Rocks.
- 20. IS: 6403 1981: Code of Practice for determination of Bearing Capacity of Shallow Foundations.
- 21. IS: 8009 1976 (Part I): Code of Practice for calculation of settlements of Foundations.
- IRC:78-2014 Standard Specifications and Code of Practice for Road Bridges Section: VIII Foundations and Substructure (Revised Edition)